

CE 34500: Transportation Engineering  
Homework 4

1. A minor road carrying 75 veh/h on each approach for eight hours of an average day crosses a major road carrying 145 veh/h on each approach for the same eight hours, forming a four-leg intersection. There are an average of two crashes per year that may be corrected by a multiway stop control. Determine whether a multiway stop sign is justified at this location. If not justified, estimate when do you need the stop sign based on traffic volume? Assume traffic growth rate is 2%.

- Total vehicular volume entering the intersection from the major approach =  $145+145 = 290$  veh/h < 300 veh/h. So, volume criteria is not satisfied.
- Total vehicular volume entering the intersection from the minor approach =  $75+75 = 150$  veh/h < 200 veh/h. So, volume criteria is not satisfied.
- 2 crashes per year < 5. Crash criteria is not satisfied.

**So, multiple stop sign is not required.**

Considering traffic growth is 2%: After 1<sup>st</sup> year, traffic on major road would be =  $290 \times 1.02 = 295.8 < 300$ ;

After 2<sup>nd</sup> year:  $295.8 \times 1.02 = 301.7 > 300$ . After 2<sup>nd</sup> year, multiple stop sign should be installed.

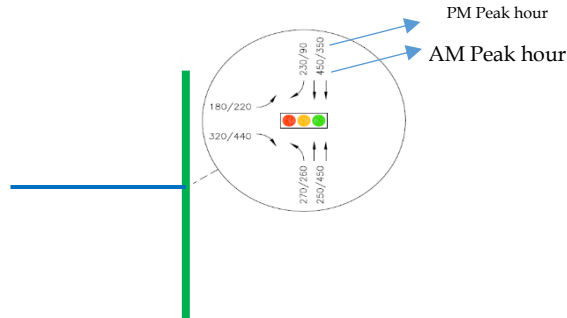
2. The table below shows 15-minute volume counts during the peak hour on an approach of an intersection. Determine the PHF and the design hourly volume of the approach.

<u>Time</u>	<u>Volume</u>
6-6:15 pm	375
6:15-6:30 pm	380
6:30-6:45 pm	412
6:45-7:00 pm	425
7:00-7:15 pm	390
7:15-7:30 pm	350

1. 6 to 7:  $V = 375+380+412+425 = 1592$  veh/hour
2. 6:15 to 7:15 = 1607 veh/hour
3. 6:30 to 7:30 = 1577 veh/hour

$$PHF = \frac{1607}{4 * 425} = 0.945$$

3. Using Webster method, determine a suitable signal timing for the following intersection. Traffic volume are given during AM/PM peak hour. Assume PHF=0.95, Saturation flow for left turn is 1615 veh/hour and through/right 3700 veh/hour. Assume numbers if not given using your engineering judgement.



#### Signal Timing for AM

Phase	Lane Group	AM Flow	Flow/PHF, q	Saturation Flow, s	q/s	Yi	Green Time
A	Left turn	180	189	1615	0.12	0.12	7.3
	Right Turn	320	337	3700	0.09		
B	Right turn	230	242	3700	0.07	0.13	7.9
	Through	450	474	3700	0.13		
C	Left turn	270	284	1615	0.18	0.18	10.9
	Through	250	263	3700	0.07		

$$\text{Total Y} = 0.43 \quad \text{Green Time} = 26$$

$$\text{Optimum cycle time} = 32.4561 \sim 35$$

$$\text{Green time} = 26$$

Time lost = 3s/phase

$$\text{Optimum cycle length} = \frac{1.5 \times 3 \times 3 + 5}{1 - 0.43} = 32.4 \text{ s} \sim 35 \text{ s}$$

#### Signal Timing for PM

Phase	Lane Group	PM Flow	Flow/PHF, q	Saturation Flow, s	q/s	Yi	Green Time
A	Left turn	220	232	1615	0.14	0.14	8.9
	Right Turn	440	463	3700	0.13		
B	Right turn	90	95	3700	0.03	0.1	6.3
	Through	350	368	3700	0.10		
C	Left turn	260	274	1615	0.17	0.17	10.8
	Through	450	474	3700	0.13		

$$\text{Total Y} = 0.41 \quad \text{Green Time} = 26$$

$$\text{Optimum cycle time} = 31.35 \sim 35$$

$$\text{Green time} = 26$$